

# **Girder Design and Site Vibration Studies for SPEAR-3**

Domenico Dell'Orco  
SLAC/SSRL





# Girder design and site vibration studies for SPEAR 3

Domenico Dell'Orco - SLAC/SSRL

22nd Advanced ICFA Beam Dynamics Workshop on Ground Motion in Future Accelerators  
November 6-9, 2000

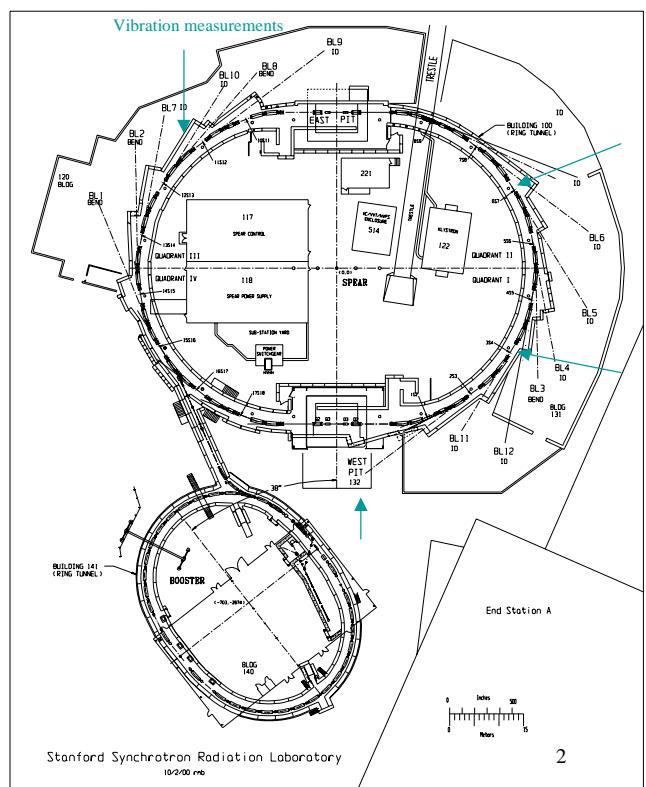
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## SPEAR 3 Upgrade

- Project Goals
  - ◆ Decrease beam emittance from 160 to 18 nm-rad
  - ◆ Increase max. stored beam current from 100 to 500 mA
  - ◆ Increase injection energy from 2.3 to 3 GeV
  - ◆ Maintain long beam lifetimes (> 20 hours)
- Requirements
  - ◆ New lattice with new magnets, power supplies, vacuum system, RF system, cabling, instrumentation and controls, and shielding modifications

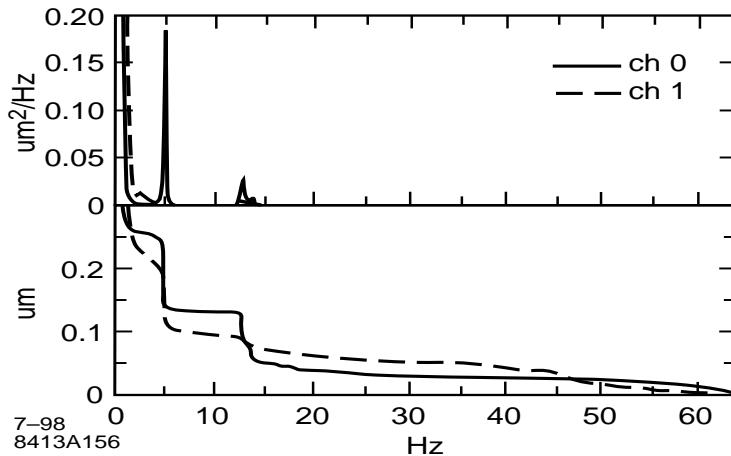
from T. Elioff - Lehman Review- June 13, 2000



# SPEAR 2 Girders

- Vibration Modes in SPEAR2 girders
  - ◆ Ground vibrations amplified by girder =  $0.04 \mu\text{m rms}$
  - ◆ Vertical motion at dipoles =  $.25 \mu\text{m rms}$  (6X)
  - ◆ Horizontal motion at dipoles =  $.75 \mu\text{m rms}$  (19X)
- Goal: increase natural frequency to  $\sim 20$  Hz

Global amplification



G. Bowden at Spear2

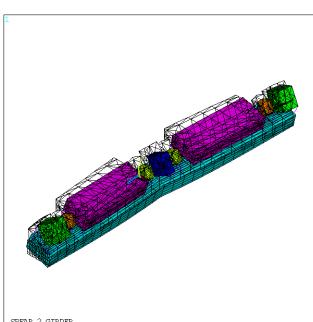
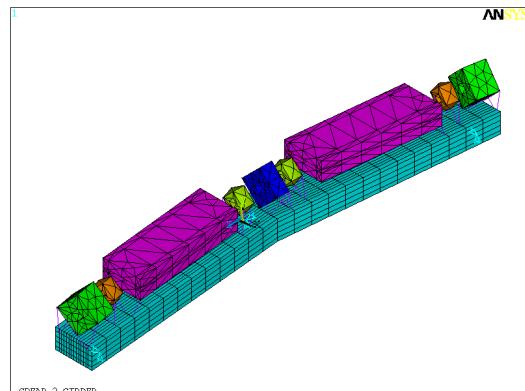
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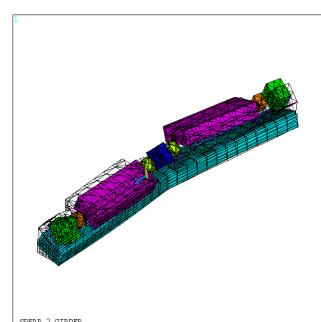
## SPEAR 2 Girder vibration modes

with flexible supports, computed with ANSYS

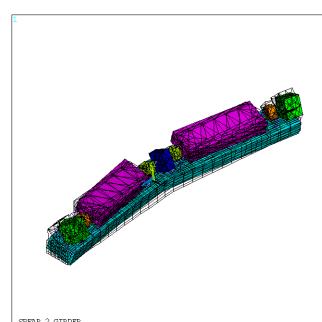
Rebars + stirrups+  
concrete + magnets



Mode 1: 4.4 Hz



Mode 2: 12.7 Hz



Mode 3: 13.46 Hz

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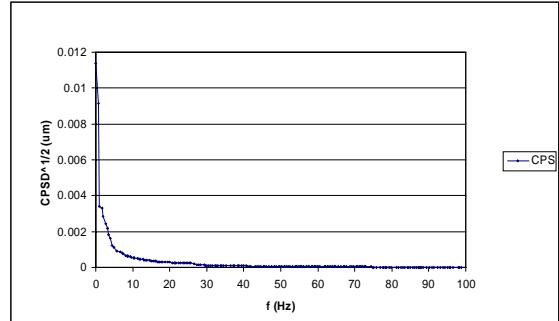
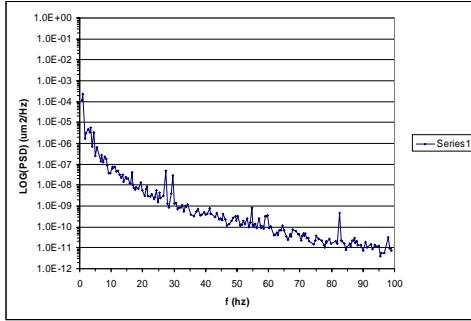
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# Ground motion spectrum and girder natural frequency

Design guideline: increase natural frequency to ~20 Hz      Why?

$f_{\text{ground}}$	5	5	5	5	5
$f_{\text{girder}}$	5	12	20	30	35
$f_{\text{ground}}/f_{\text{girder}}$	1	0.42	0.25	0.17	0.14
$\xi/\xi_c$	0.02	0.02	0.02	0.02	0.02
A	25.02	1.21	1.07	1.03	1.02

$f_{\text{ground}}$	12	12	12	12	12
$f_{\text{girder}}$	5	12	20	30	35
$f_{\text{ground}}/f_{\text{girder}}$	2.4	1	0.6	0.40	0.34
$\xi/\xi_c$	0.02	0.02	0.02	0.02	0.02
A	0.21	25.02	1.56	1.19	1.13



Measured at SPEAR in 1982 by Jendrzejczyk, Smith, Wambganss, Zhu

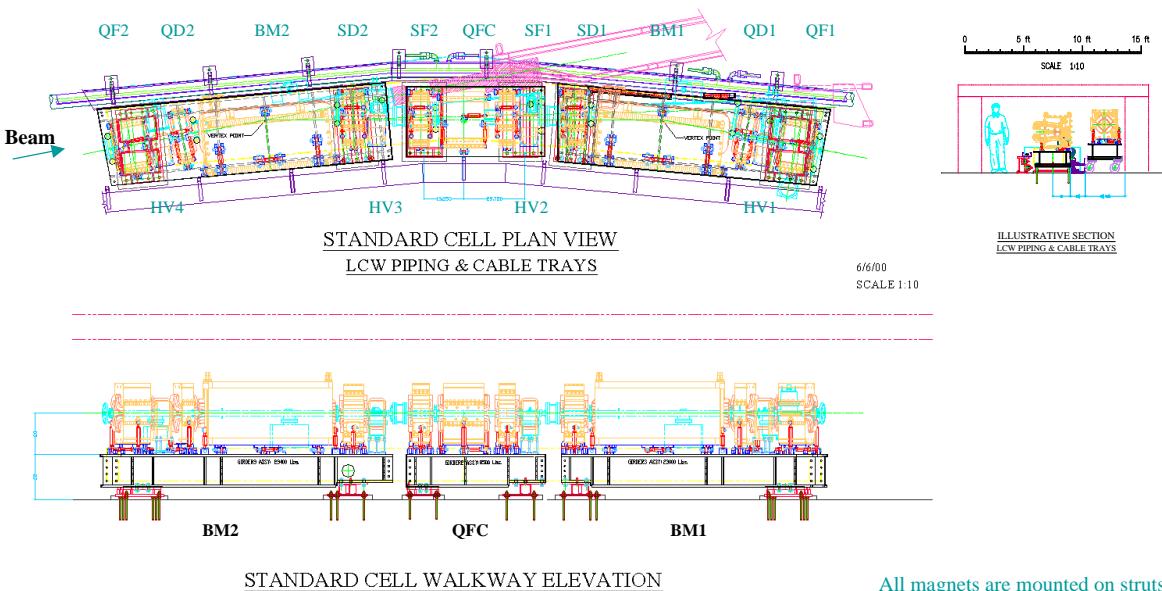
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## SPEAR 3 Magnet supports



### 14 Standard cell girders (BM1, BM2, QFC)

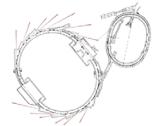


SPEAR 3 steel girders with beam at 0.508 m (20") from top surface. Concrete floor.

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# SPEAR 3 Magnet supports

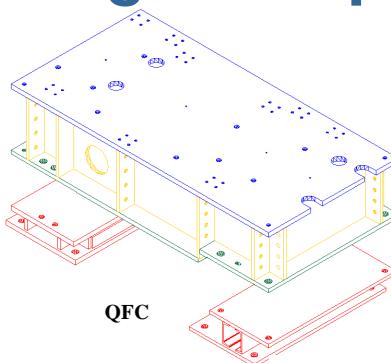


Thermal expansion at flexible joint:  
12.  $\mu\text{m}/\text{m}/\text{C}$  \* 3 m \* 30 C = 1mm

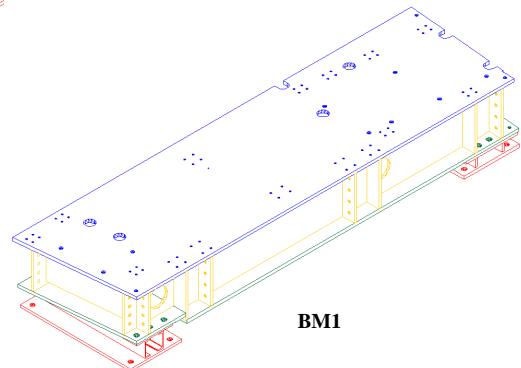
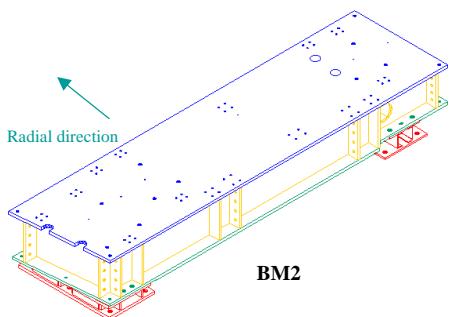
1" thick top/bottom plates  
3/4" thick all other plates

Welded and stress relieved structure

Concrete floor  
fixed girders  
adjustable magnets



Stiffeners to redistribute static loads and stiffen the girder in the radial direction (the welds can act like hinges especially for small deflections).



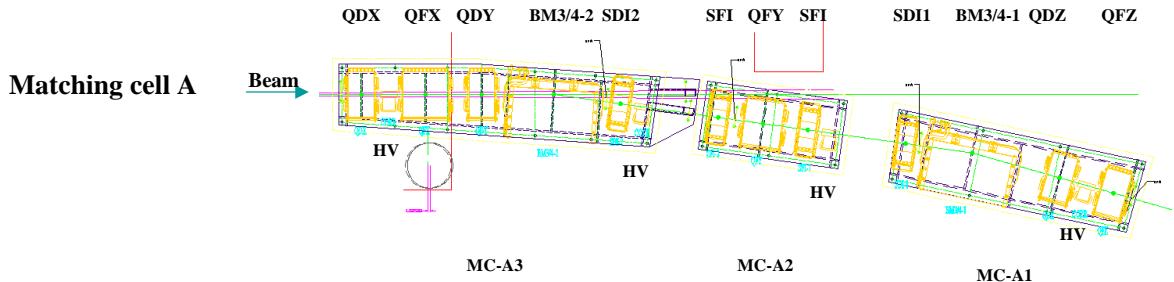
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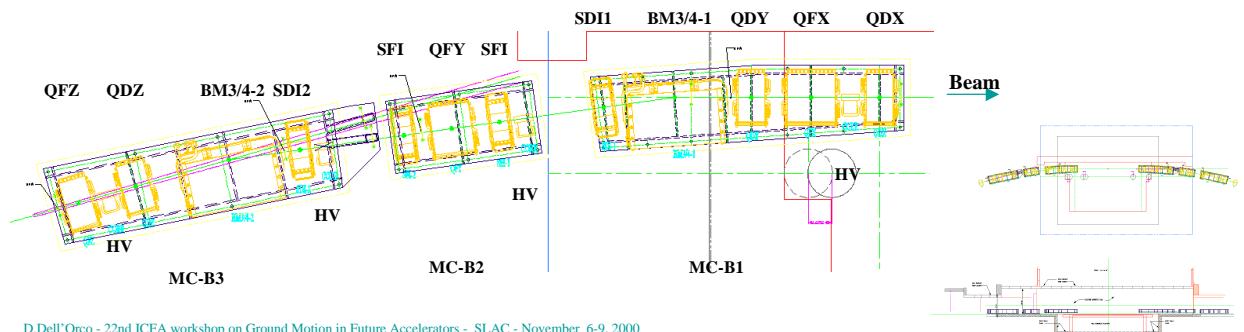
# SPEAR 3 Magnet supports



2 Matching cells A + 2 Matching cells B



**Matching cell B**



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# SPEAR 3 Girders

	Qty	109D	145D	15Q	34Q	50Q	60Q	21S	25S	HV
BM1	14			1	1				1	1
QFC	14					1		2		1
BM2	14		1	1	1				1	2
MC-A1	2	1			2			1		1
MC-A2	2					1		2		1
MC-A3	2	1			2		1	1		2
MC-B1	2	1			2		1	1		1
MC-B2	2					1		2		1
MC-B3	2	1			2			1		2
Total		8	28	28	44	18	4	44	28	72
<b>Mag. Weight (lb)</b>		10581	13773	1003	2191	3193	3818	1041	1232	400

Girder	Length (in)	Width (in)	Height (in)	Girder weight (lb)	Magnet weight (lb)	Chamber weight (lb)	Total weight (lb)
BM1	133.5	38	16	4290	18600	1490	24400
QFC	67	34	16	2153	5700	750	8600
BM2	141	38	16	4531	19000	1570	25100
MC-A1	134	38.5	15	4306	16400	1500	22200
MC-A2	74	36	15	2378	5700	830	8900
MC-A3	190	37	15	6106	20600	2120	<b>28800</b>
MC-B1	155	37	15	4981	20200	1730	26900
MC-B2	73	36	15	2346	5700	810	8900
MC-B3	167	38.5	15	5367	16800	1860	24000

Matching cell A3 is longer and heavier than all the other girders =>  
highest forces in the earthquake analysis and lowest vibration frequency

## SPECIFICATION FOR SEISMIC DESIGN OF BUILDINGS, STRUCTURES, EQUIPMENT and SYSTEMS AT THE STANFORD LINEAR ACCELERATOR CENTER, August 5, 1999

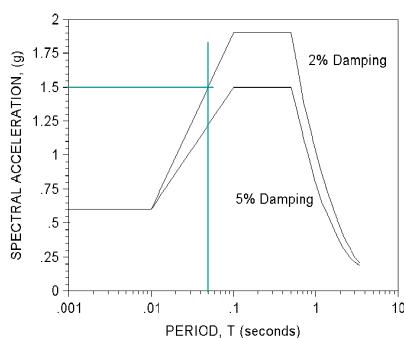


Figure 1. Response Spectra for Mechanical Systems  
Horizontal Motions

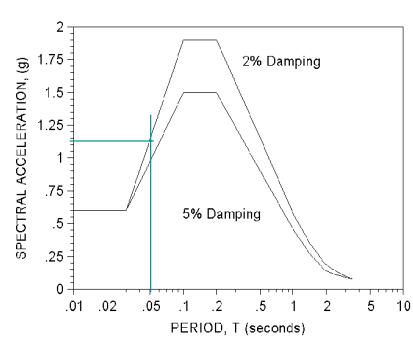


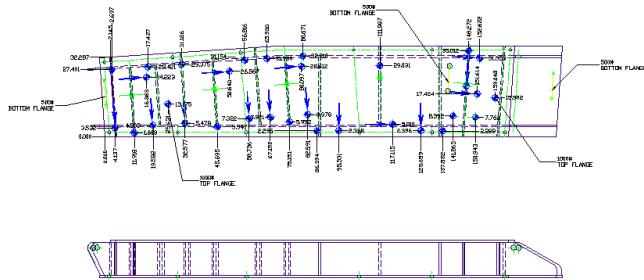
Figure 1. Response Spectra for Mechanical Systems  
Vertical Motion

Horizontal acceleration: 1.5 g

Vertical acceleration: 1.15 g

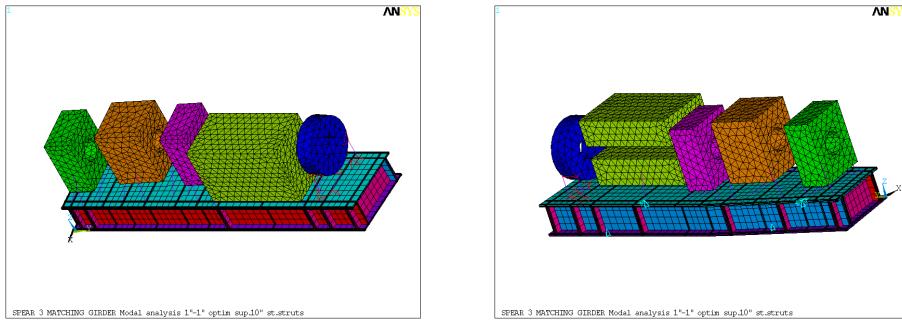
Assumption of 20 Hz natural frequency for girder/magnets system (verified later)

# ANSYS model of MC-A3



#### **OLD version of MC-A3 used for the ANSYS model**

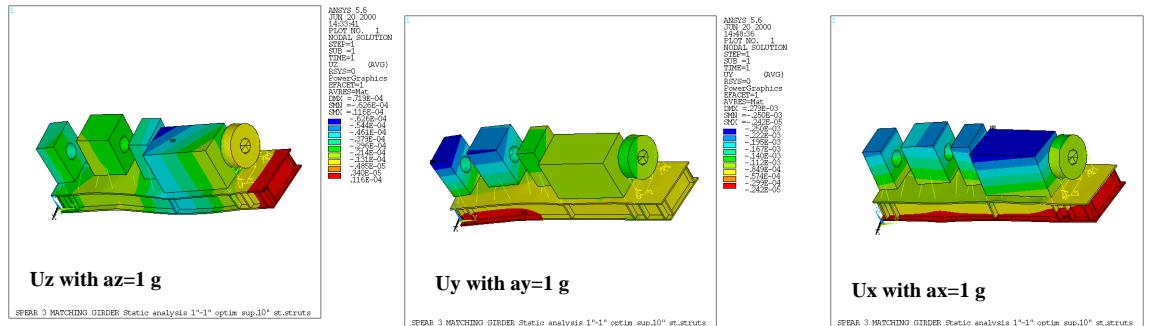
4 girder support points, realistic magnets, rigid struts



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# ANSYS static analysis of MC-A3



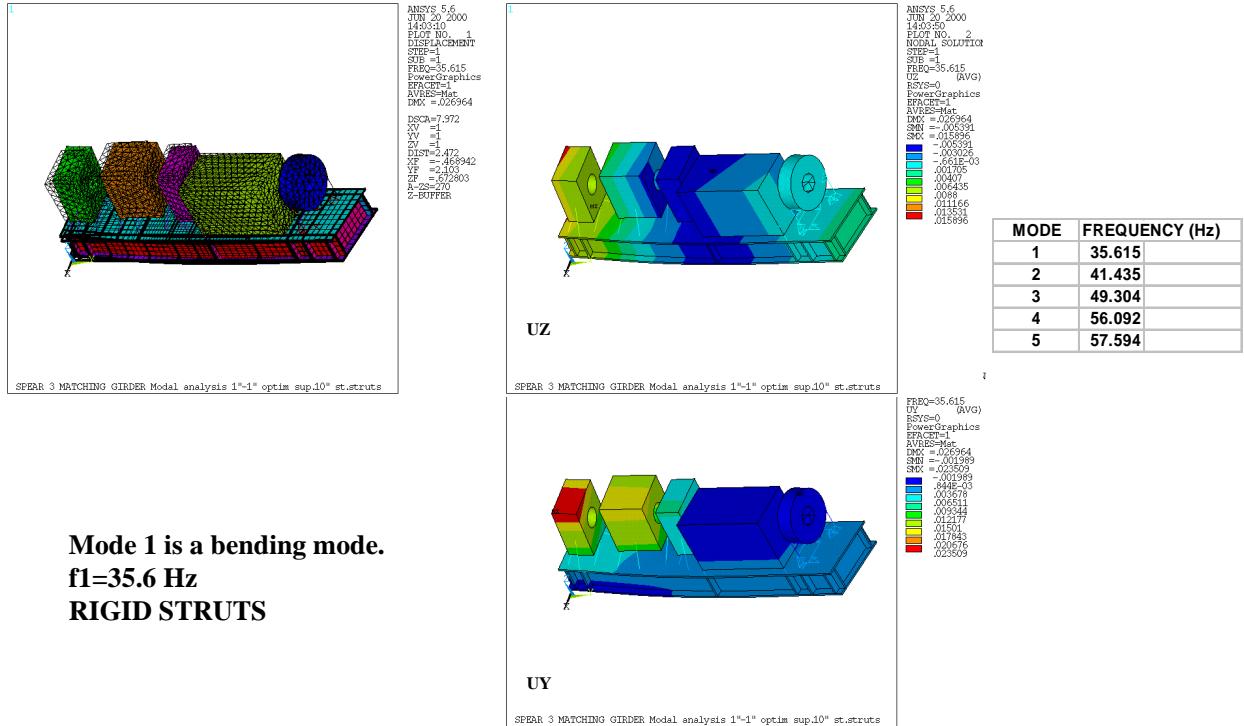
**Dead weight only:**  $Uz_{max}=0.07$  mm  
**Dead weight +  $az=1.15$  g:**  $Uz_{max}=0.15$  mm

**ay=1.55 g; Uy<sub>max</sub>=0.43 mm**

$$ax=1.55 \text{ g; } Ux_{\max}=0.34 \text{ mm}$$

**These deflections are computed assuming that the strut stiffness is very high.**

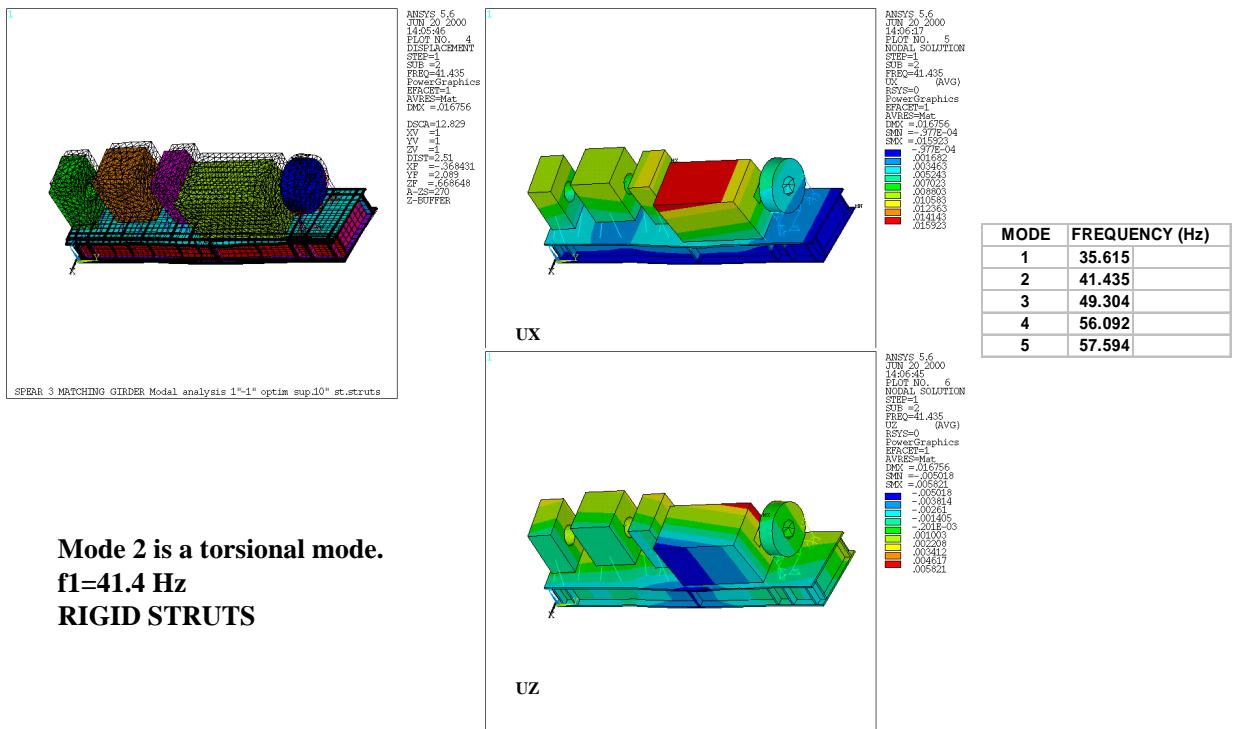
# ANSYS modal analysis of MC-A3 - Mode 1



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# ANSYS modal analysis of MC-A3 - Mode 2

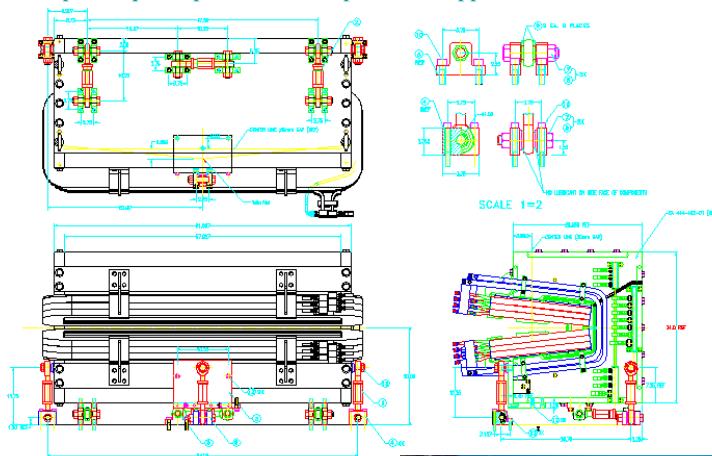


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# SPEAR 3 Gradient dipole

Dipoles, quadrupoles and sextupoles are supported with 6 struts



max load: 22000 lb - 10000 kg  
during design earthquake

**breakaway torque**

## collars

deflection: 0.0005-0.002 in/4000 lb  
13-51 µm/1800 kg  
unloaded horizontal struts



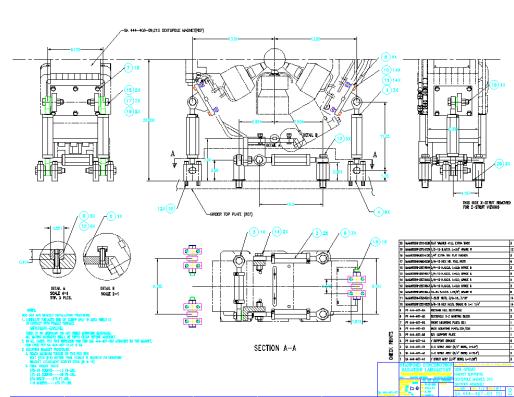
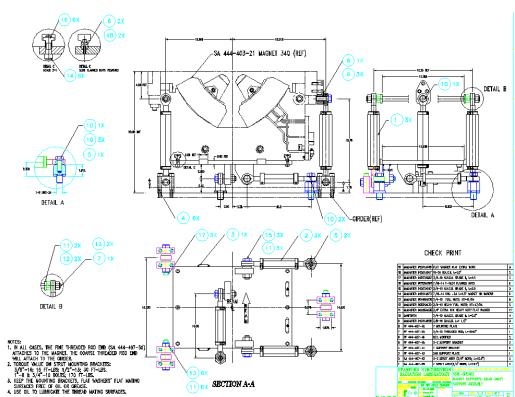
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# SPEAR 3 Quadrupole and Sextupoles

Dipoles, quadrupoles and sextupoles are supported with 6 struts



# Magnet supports

Magnets with struts on rigid floor

DD- 8/23/00 - BM1 girder			Strut stiffness		Strut stiffness	
25S only	Mode	ANSYS	MathCad	Diff %		0.5mil/4000lb
145D only	1	24.614	24.826	-0.85	4th	35.2
	2	60.169	60.433	-0.44		85.7
	3	63.336	63.437	-0.16		89.4
	4	113.862	114.573	-0.62		162.9
	5	167.499	170.802	-1.93		240.3
	6	221.441	223.177	-0.78		315.6
15Q only	Mode	ANSYS	MathCad	Diff %		
34Q only	1	23.716	24.024	-1.28	3rd	33.8
	2	52.304	52.367	-0.12		73.7
	3	67.958	69.01	-1.52		97.0
	4	137.643	139.275	-1.17		195.8
	5	179.626	184.099	-2.43		261.7
	6	245.385	247.676	-0.92		348.4
Fixed girder	Mode	ANSYS	MathCad	Diff %		
BM1 with stiff struts						
Bm1 with real struts	1	33.87				
	2	44.60				
	3	47.03				
	4	55.64				
	5	69.46				
	6	71.39				
25mm/mm max	Mode	ANSYS				
52mrad/mm max	1	17.17				
	2	18.64				
	3	22.67				
	4	22.88				
	5	24.75				
	6	32.88				

What about amplitudes?

Rigid mounts are assumed to be 50 times stiffer than struts

Magnets with rigid struts on girder

Magnets with real struts on girder

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## Requirements - Acceptable values

Magnet-to-orbit amplification Factors for SPEAR3 (J. Corbett 10/21/2000, SLAC)

- Uncorrelated motion of magnet on girders:
  - ◆ Lattice-to-beam Vertical amplification = ~30
  - ◆ Lattice-to-beam Horizontal amplification = ~40
 Uncorrelated motion of magnet on girders
- Magnet motion of ganged to motion of full cell:
  - ◆ Lattice-to-beam Vertical amplification = ~5
  - ◆ Lattice-to-beam Horizontal amplification = ~10
- Allowable uncorrelated magnet motion (10% of beam size /Amplification Factor):
  - ◆ Vertical motion =  $1 \mu\text{m} / 30 = 33 \text{ nm}$
  - ◆ Horizontal motion =  $10 \mu\text{m} / 40 = 250 \text{ nm}$
- These are allowable movements of individual magnets supported with struts on a girder.

Before feedback  
Which frequency range?

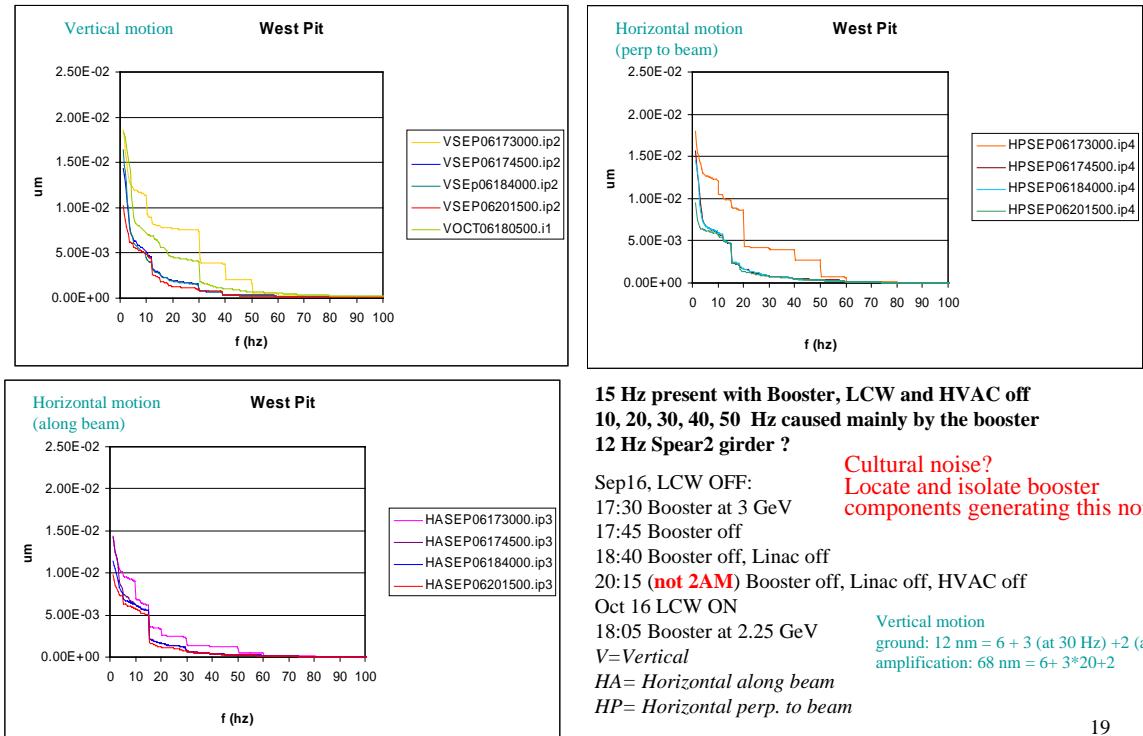
Integrated rms motion of Spear2 quadrupoles: vert. 70 nm, horiz. 700 nm

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# Ground motion measurement at the West Pit

Cumulative rms displacement vs. Frequency with Mark L-4 1Hz Measured by Andrei Seryi

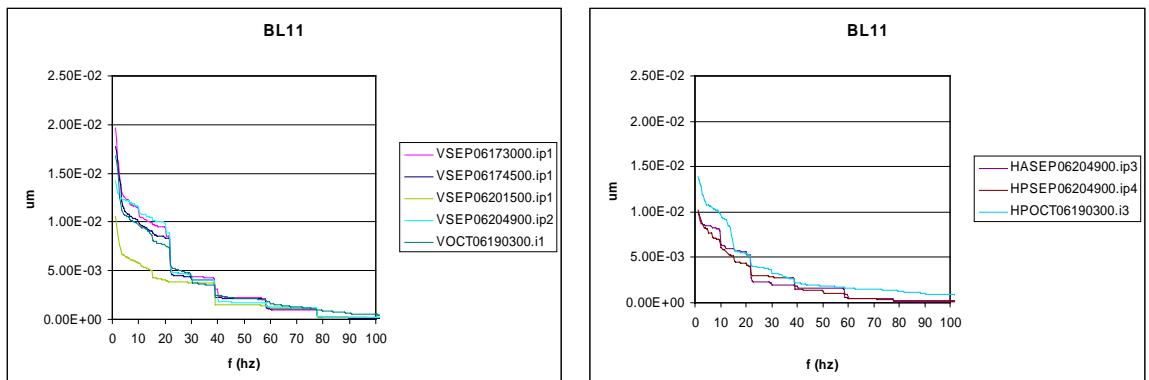


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# Ground motion measurement at BL11 (concrete pad)

Cumulative rms displacement vs. Frequency



Sep16, LCW OFF:  
17:30 Booster at 3 GeV  
17:45 Booster off  
18:40 Booster off, Linac off  
20:15 Booster off, Linac off, HVAC off  
20:49 Booster on, Linac on, HVAC on

Oct 16 LCW ON  
19:03 Booster at 2.25 GeV

V=Vertical  
HA= Horizontal along beam  
HP= Horizontal perp. to beam

15, 40 Hz present with Booster, LCW and HVAC off  
22 Hz caused by HVAC

10, 20, 30, 40, 50 Hz cause mainly by the booster

LCW, horiz. 15 Hz and less?

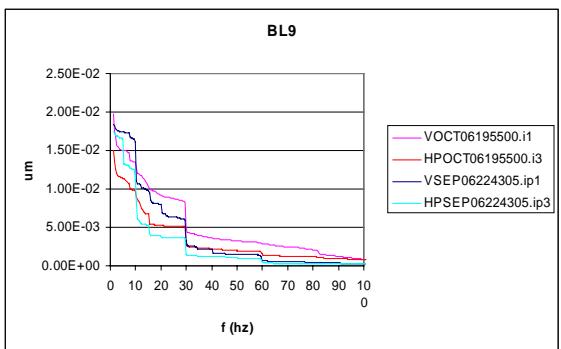
LCW, vert., almost no effect?

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# Ground motion measurement at BL5 & BL9 (concrete pad)

Cumulative rms displacement vs. Frequency



15 Hz present with Booster, LCW and HVAC off ??

Sep16, LCW OFF, Booster at 3GeV:  
Oct 16 LCW ON, Booster at 2.25 GeV

V=Vertical

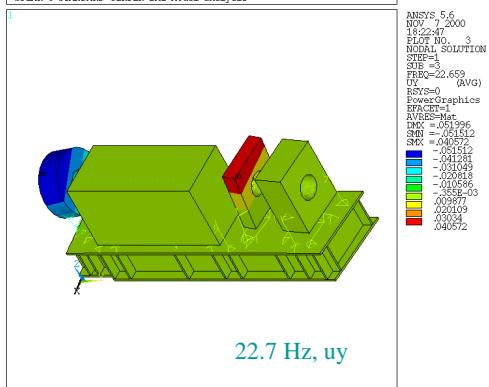
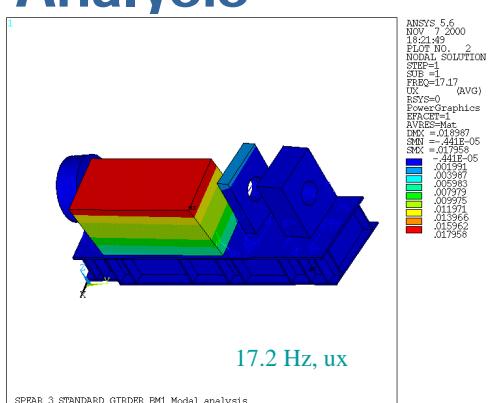
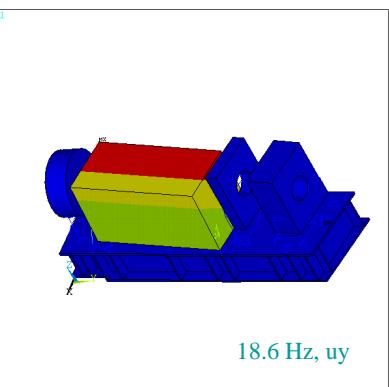
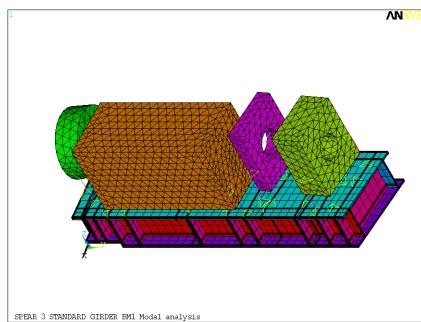
HA= Horizontal along beam

HP= Horizontal perp. to beam

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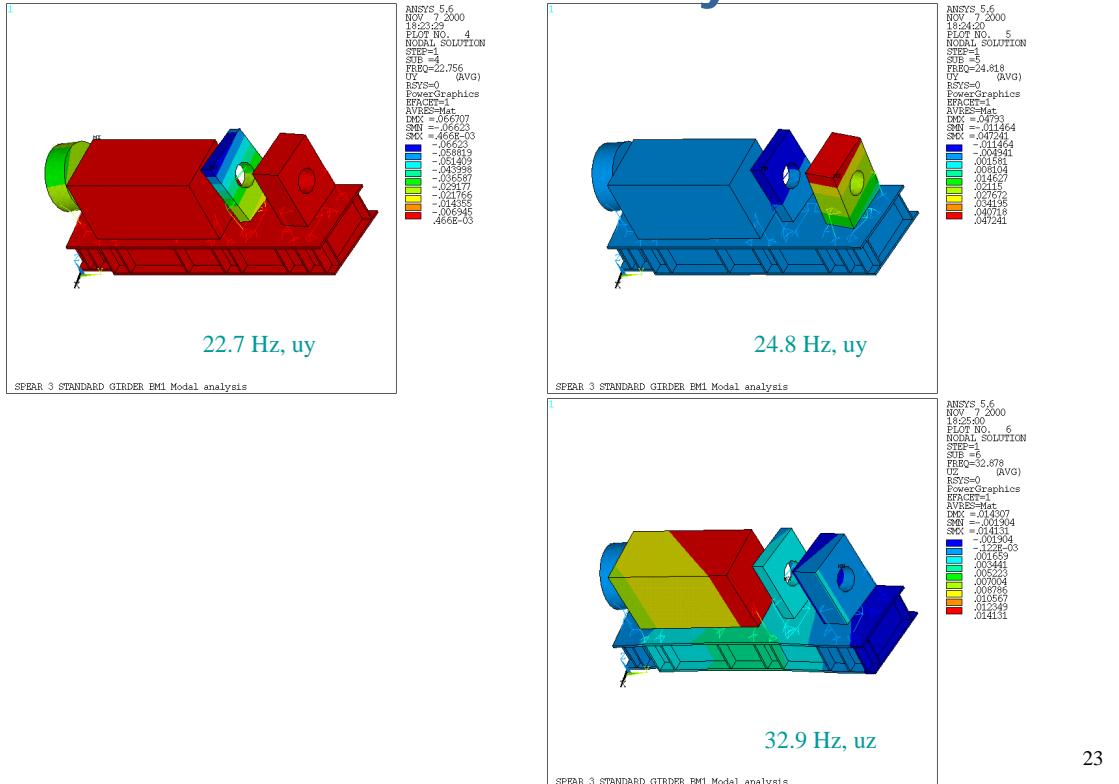
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## Girder Bm1 modal Analysis



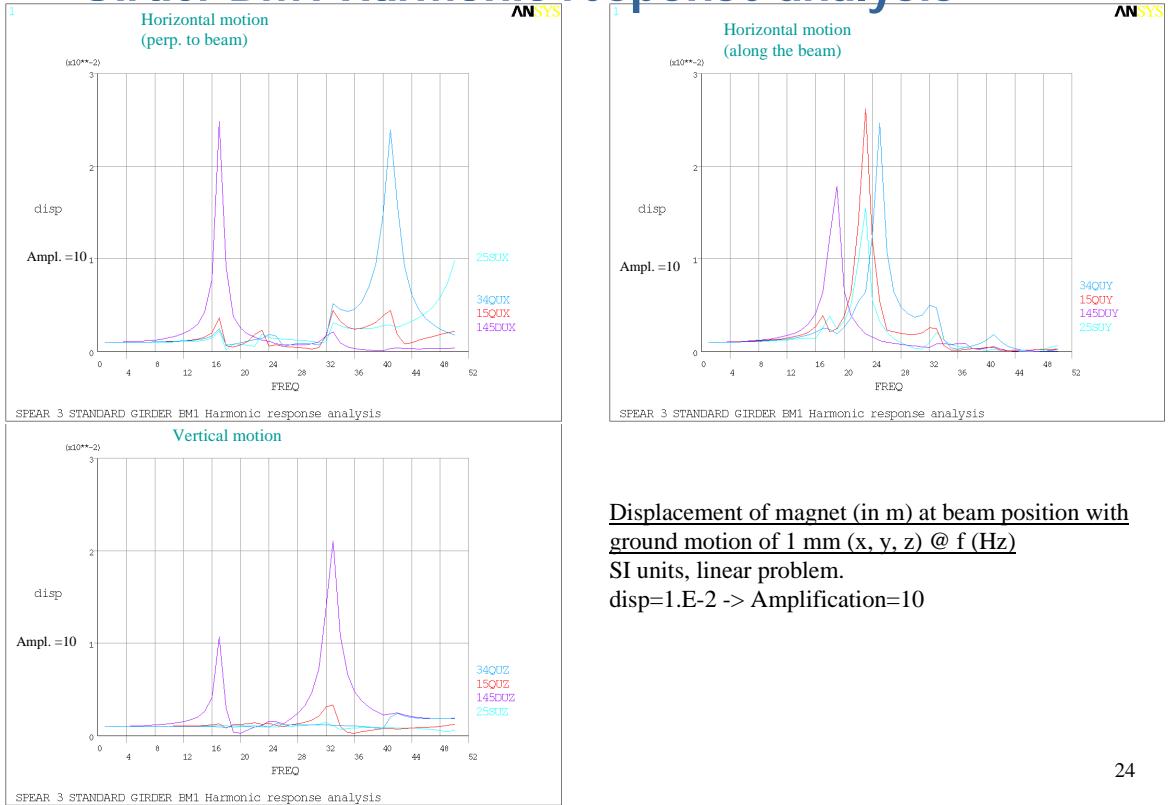
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# Girder Bm1 modal Analysis



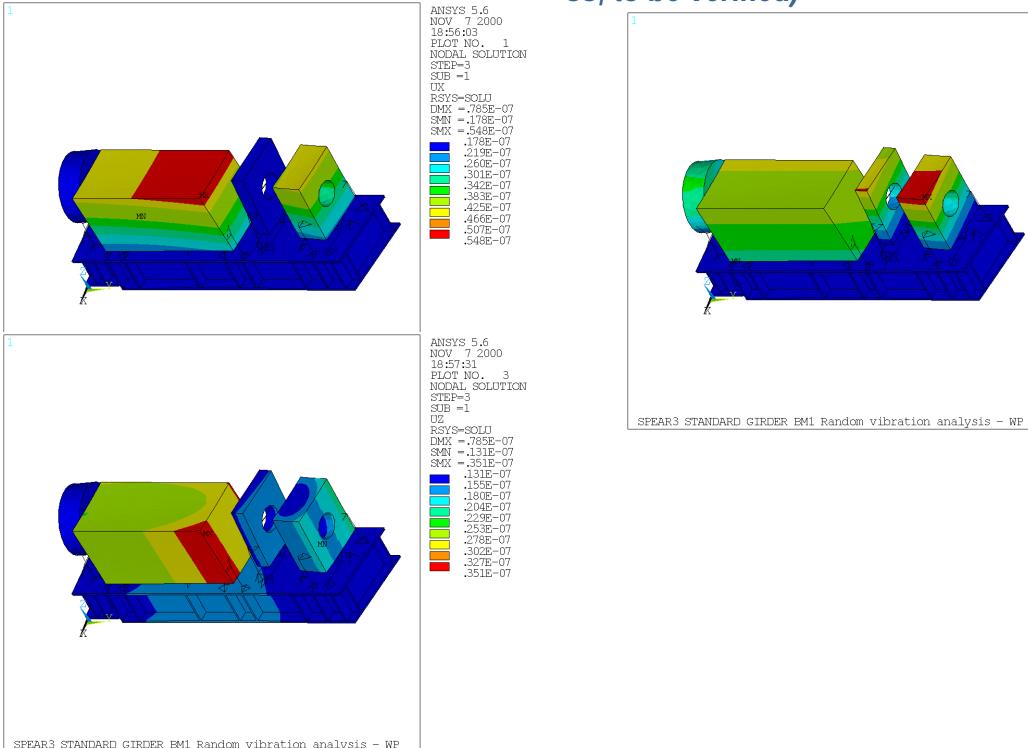
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# Girder Bm1 Harmonic response analysis



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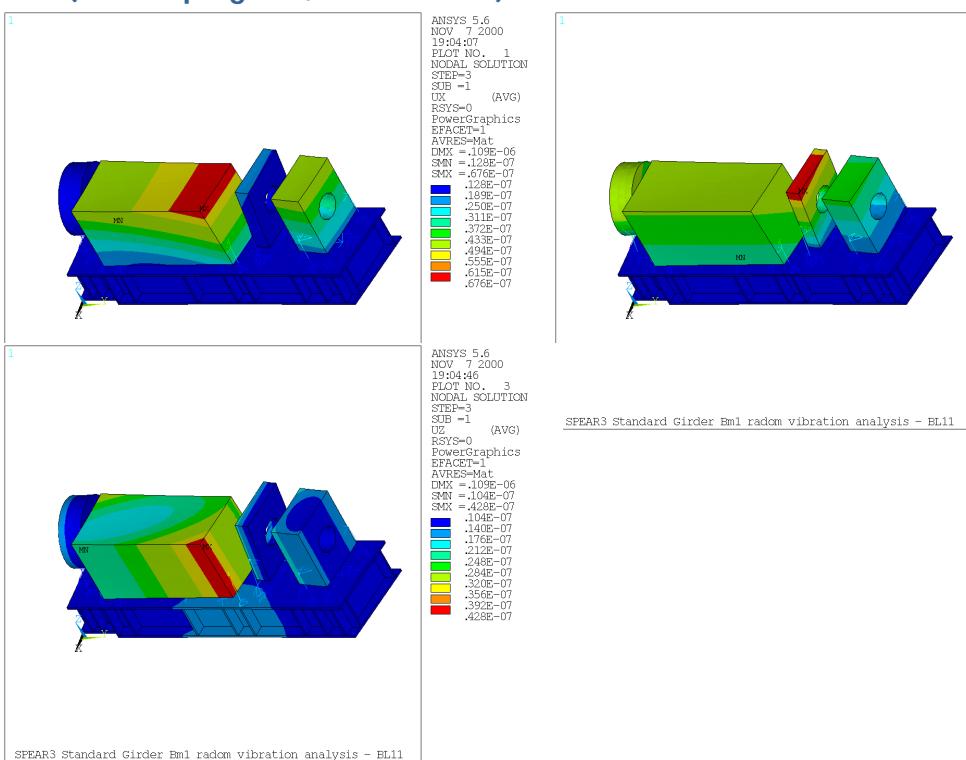
# Girder Bm1 Random vibration analysis at the West Pit (work in progress, to be verified)



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# Girder Bm1 Random vibration analysis at BL11 (work in progress, to be verified)

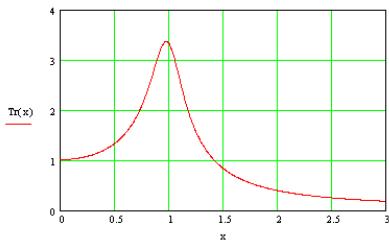


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# Simplified analysis of vertical damper

$A := 50 \cdot 10^{-9} \text{ m}$		Amplitude
$f := 20 \text{ Hz}$		Frequency
$\omega := 2 \cdot \pi \cdot f$	$\omega = 125.664 \text{ Hz}$	
$m_s := 6350 \text{ kg}$		mass
$v := A \cdot 2 \cdot \pi \cdot f$	$v = 6.283 \cdot 10^{-6} \frac{\text{m}}{\text{s}}$	max speed
$E_m := \frac{m_s \cdot v^2}{2}$	$E_m = 1.253 \cdot 10^{-7} \text{ J}$	magnet stored energy energy
$L_f := 0.7$		loss factor
$G := 30 \cdot \frac{1}{145} \cdot 10^6 \text{ N m}^{-2}$	$G = 2.069 \cdot 10^5 \text{ Pa}$	Shear modulus
$A_r := 0.05 \cdot 0.20 \text{ m}^2$		Area
$th := 0.002 \cdot 25.4 \cdot 10^{-3} \text{ m}$		Thickness
$nd := 4$		Number of dampers
$Ed := \frac{G \cdot \left(\frac{A}{th}\right)^2 \cdot 2 \cdot A_r \cdot th \cdot L_f \cdot nd}{2}$	$Ed = 2.831 \cdot 10^{-7} \text{ J}$	Energy dissipated by shear in half cycle
$\text{ratio} := \frac{Ed}{E_m}$	$\text{ratio} = 2.274$	
$S_f := G \cdot A_r \cdot \frac{A \cdot 2 \cdot nd}{th}$	$S_f = 16.291 \text{ N}$	Shear force
$k := \frac{4000 \cdot 3}{2.2} \cdot \frac{9.81}{2.0002 \cdot 0.0254} \frac{\text{N}}{\text{m}}$	$k = 5.267 \cdot 10^8 \frac{\text{kg}}{\text{s}^2}$	Strut stiffness
$C_{cr} := 2 \sqrt{k \cdot m_s}$	$C_{cr} = 3.657 \cdot 10^6 \frac{\text{kg}}{\text{s}}$	Critical damping
$C := \frac{Ed \cdot 2}{A^2 \cdot \omega \cdot \pi}$	$C = 5.777 \cdot 10^5 \frac{\text{kg}}{\text{s}}$	
$\zeta := \frac{C}{C_{cr}}$	$\zeta = 0.158$	Damping ratio
$Tr(x) := \frac{\sqrt{1 + (2 \zeta x)^2}}{\sqrt{(1 - x^2)^2 + (2 \zeta x)^2}}$	$Tr(1) = 3.32$	
		$\frac{Y}{y_0}$



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